

1 **4.0 MONITORED NATURAL RECOVERY**

2

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## 4.0 MONITORED NATURAL RECOVERY

### 4.1 INTRODUCTION

Monitored natural recovery (MNR) is a sediment cleanup method that uses ongoing, naturally occurring processes to contain, destroy, or otherwise reduce the bioavailability or toxicity of contaminants in sediment. These processes may include multiple physical, biological, and chemical mechanisms that act together to reduce the risk posed by the contaminants. Depending on the contaminants and the sediment environment, this risk reduction may occur in one or more of the following ways:

- Exposure levels are reduced by a decrease in contaminant concentration levels in the near-surface sediment zone through burial or mixing-in-place with cleaner sediment;
- The contaminant is converted to a less toxic form through destructive processes, such as biodegradation or abiotic transformations; or
- Contaminant mobility and bioavailability are reduced through increased sorption to the sediment matrix.

MNR is a new term for a sediment remedy that currently is being called by a number of different names including monitored natural attenuation (MNA) and monitored natural processes (MNP). As defined in this guidance, MNR is similar in some ways to the MNA remedy previously defined for ground water and soils (U.S. EPA 1999d). Two key differences between MNA and MNR are in the type of processes at work and the extent to which risks during remedy implementation are controllable.

The major components of a MNR remedy generally include the following:

- A detailed understanding of the natural processes that are affecting sediment and contaminants at the site;
- Conceptual and predictive modeling (either using computer modeling or another method) to predict effects of those processes in the future;
- A means to control any significant ongoing contaminant sources;
- A means to control contaminant exposure during the recovery period (to the extent possible); and
- The ability to monitor the natural processes and their effects to see if recovery is occurring.

MNR has been selected as a cleanup method for contaminated sediment at about a dozen Superfund sites so far. It usually (although not always) is combined with dredging or in-situ capping of other areas of a site. In general, project managers should evaluate each of the three major cleanup methods: MNR, capping, and removal through dredging or excavation, at every sediment site at which they may be appropriate. Some consider that all sediment site remedies are using “natural recovery”

1 because natural processes are ongoing whether or not an active cleanup is underway (e.g., NRC 2001).  
2 Although it is true that natural processes will continue to reduce residual risk at many sites, it is important  
3 for project managers to distinguish whether they are relying upon MNR to reduce risk to an acceptable  
4 level (i.e., using MNR as a remedy), or simply noting the fact that natural processes are ongoing at a site.  
5 The key factors that distinguish MNR as a remedy are the presence of unacceptable risk, the ongoing  
6 burial or degradation/transformation of the contaminant, and the establishment of a cleanup level that  
7 MNR is expected to meet in a particular time frame. Although partial natural recovery has been noted in  
8 many areas (e.g., decreases in contaminant levels in fish), there is not yet a body of literature  
9 documenting complete recovery. However, monitoring results from some areas are promising (e.g., U.S.  
10 EPA 2001f, U.S. EPA 2001g, Swindoll et al. 2000).

11  
12 Unless biodegradation or transformation is a major process at the site, MNR generally does not  
13 meet the CERCLA §121 (b)(1) preference for treatment. When contaminants left in place are above  
14 levels that allow for unlimited use and unrestricted exposure, a five year review generally is necessary  
15 (U.S. EPA 2001h).

16  
17 Highlight 4-1 lists some of the general site conditions which may be appropriate for consideration  
18 of MNR.

19  
20  
21 **Highlight 4-1: General Site Conditions Appropriate for Monitored Natural Recovery**

22 Generally, MNR may be an effective cleanup method in a variety of environments within a site, especially where:

- 23 • Sources are controlled
- 24 • Contaminant concentrations in the biologically active zone of the sediment are near risk-based cleanup
- 25 goals and declining
- 26 • Contaminant concentrations in biota are near risk-based goals and declining
- 27 • Institutional controls effectively restrict human exposure
- 28 • Natural recovery processes have a high degree of certainty to continue at rates which will contain,
- 29 destroy, or reduce the bioavailability or toxicity of contaminants within an acceptable time frame
- 30 • The sediment bed is stable and likely to remain stable
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37  
38 **4.2 ADVANTAGES AND DISADVANTAGES**

39  
40 Two advantages of MNR are its relatively low cost and its non-invasive nature. Costs associated  
41 with MNR are primarily associated with monitoring, but may also include the cost of implementing  
42 institutional controls, public education to increase the effectiveness of those controls, and, where  
43 applicable, a possible separate cost of a contingency remedy. MNR involves minimal disruption to the  
44 existing biological community, which may be an important advantage for some wetlands or sensitive  
45 environments where the harm to the ecological community due to sediment disturbance may outweigh the  
46 risk reduction of an active cleanup.

47  
48 Other advantages of MNR include the fact that no construction or infrastructure is needed, and it  
49 is therefore much less disruptive of communities than active cleanup methods such as dredging or in-situ

1 capping. No property is needed for materials handling, treatment, or disposal facilities, and no  
2 contaminated materials are transported through communities.

3  
4 Two disadvantages of MNR are that it generally leaves contaminants in place and that it can be  
5 slow in comparison to active cleanup methods. When recovery is based on natural burial, contaminants  
6 may be re-exposed or dispersed if the sediment bed is significantly disturbed by unexpected natural or  
7 man-made forces. Although biological and chemical transformation of contaminants can occur, these  
8 processes are frequently too slow to provide for remediation in a reasonable time frame for the persistent  
9 contaminants of concern at many Superfund sites.

10  
11 Other disadvantages of MNR include the fact that it may have a higher level of uncertainty  
12 associated with it than active cleanup methods and the need to rely upon institutional controls. Major  
13 areas of uncertainty frequently include the ability to predict future sedimentation rates in dynamic  
14 watersheds. Remedies which include MNR frequently rely upon institutional controls, such as fish  
15 consumption advisories, to control human exposure during the recovery period. These controls may have  
16 limited effectiveness and have no ability to control ecological exposures.

### 17 18 **4.3 NATURAL RECOVERY PROCESSES**

19  
20 The natural processes of interest for MNR include a variety of processes that, under favorable  
21 conditions, act without human intervention to reduce the mass, toxicity, mobility, volume, or  
22 concentration of contaminants in the sediment bed. These natural processes include the following:

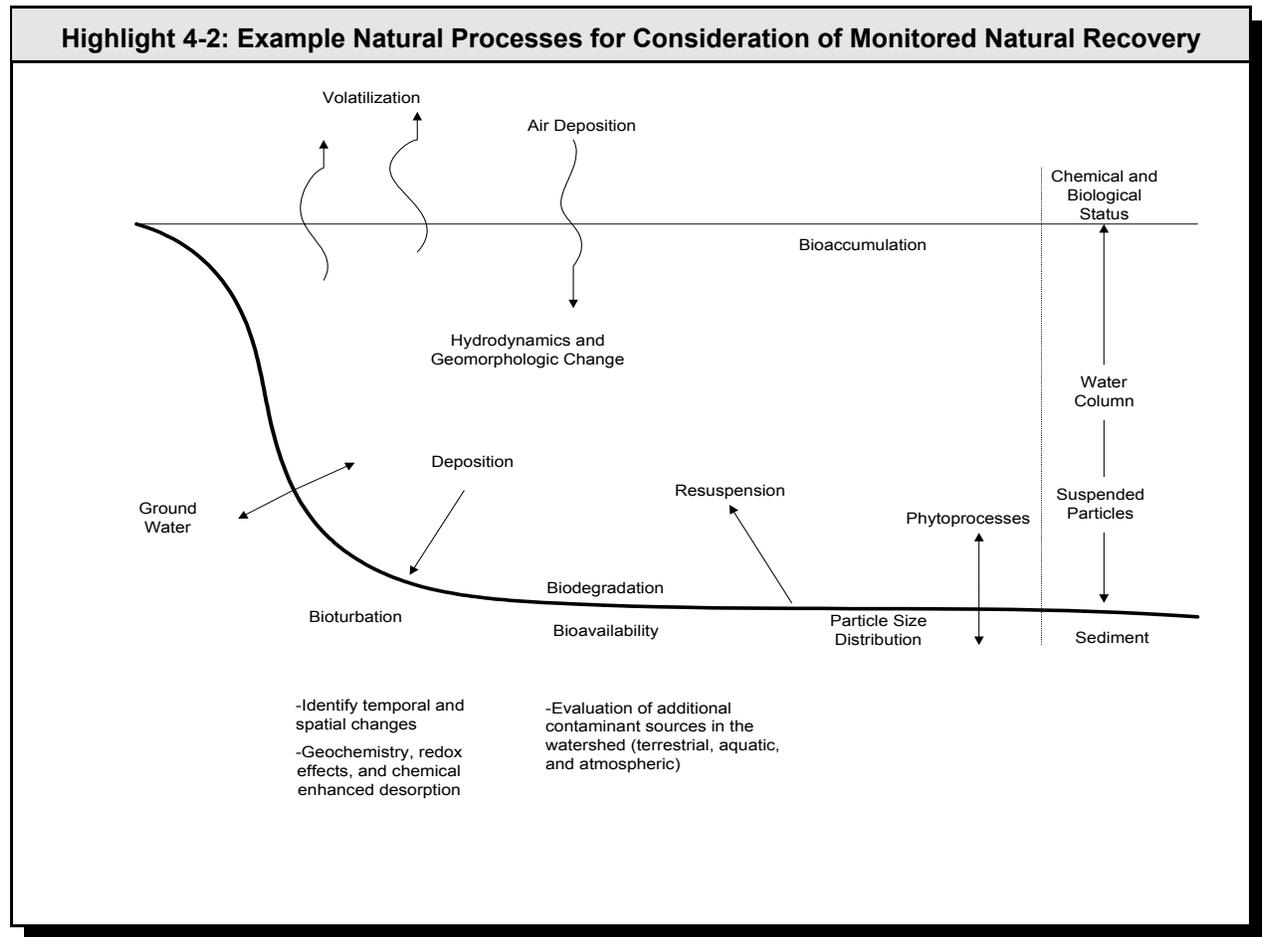
- 23  
24 • Physical processes (e.g., sedimentation, advection, diffusion, dilution, bioturbation,  
25 volatilization);  
26  
27 • Biological processes (e.g., biodegradation, biotransformation, phytoremediation,  
28 biological stabilization); and  
29  
30 • Chemical processes (e.g., oxidation/reduction, stabilization, sorption).

31  
32 Highlight 4-2 illustrates some of the natural processes that the project manager should consider  
33 during evaluation of MNR. With few exceptions, these processes interact in aquatic systems, sometimes  
34 reducing risks and at other times, increasing them. For example, as recognized by the U.S.  
35 Environmental Protection Agency's (EPA) Science Advisory Board (SAB) Environmental Engineering  
36 Committee, *Monitored Natural Attenuation: USEPA Research Program - An EPA Science Advisory  
37 Board Review* (U.S. EPA 2001i), sustained burial processes can impede certain degradation processes,  
38 such as aerobic biodegradation. Understanding the interactions between effects and prioritizing the  
39 significance of these effects to the MNR remedy should be part of a natural process analysis.

#### 40 41 **4.3.1 Physical Processes**

42  
43 Physical processes do not change the chemical nature of contaminants. Instead, physical  
44 processes may bury, mix, dilute, or move contaminants to another medium. Physical processes of  
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interest for MNR include sedimentation, erosion, re-suspension, diffusion, dilution, bioturbation, advection, and volatilization (including temperature-induced desorption of semi-volatiles). All of these processes may reduce contaminant concentrations in surface sediment. However, of these processes, only sedimentation (i.e., deposition and consolidation) reduces risk by containing contaminants in place. Erosion, dispersion, dilution, bioturbation, advection, and volatilization also may reduce contaminant concentrations in sediment, but at the expense of moving the contaminants to another medium or dispersing them over a wider area (e.g., via ground water or surface water). These processes may reduce or increase the risk posed by the contaminants.

Physical processes in sediment operate at vastly different rates. Some are continuous processes, others seasonal or episodic. In general, processes in which contaminants are transported by bulk movement of particles or pore water (e.g., erosion, bioturbation, advection) occur at faster rates than processes in which contaminants are transported by diffusion or volatilization and, therefore, are generally more important when evaluating MNR. Processes that result in particle movement are particularly important for hydrophobic or other contaminants that are strongly sorbed to sediment particles.

1 In high-energy environments, sediment transport and deposition are likely to be the dominant  
2 factors in contaminant movement and burial, and thus it is likely that the sediment transport process will  
3 need to be adequately characterized in such environments. In low-energy environments, sedimentation  
4 and bioturbation are likely to dominate contaminant redistribution processes.  
5

6 Hydraulic transport (i.e., advection and dispersion) of cleaner sediment in the watershed may lead  
7 to natural burial of contaminated sediment in a depositional environment. Natural burial may reduce the  
8 availability of the contaminants to aquatic plants and animals and, therefore, may reduce  
9 bioaccumulation. Particles of all size ranges deposit in quiescent water, forming layers of sediment.  
10 These particles with time consolidate, expelling pore water. The process, called bed accretion, results in  
11 the burial of the contaminated sediment by clean sediment (i.e., sediment from uncontaminated or  
12 remediated portions of the watershed). The overlaying cleaner sediment serves to reduce the flux of  
13 contaminants into the surface water by creating a longer pathway that the desorbed contaminants must  
14 travel to reach the sediment bed/water column interface.  
15

16 The protectiveness provided by sedimentation depends upon the stability of the new sediment  
17 bed, which is mainly related to the shear stresses to which the deposited materials are subjected. Major  
18 events, such as severe storms or flood events, may scour the buried sediment, exposing contaminated  
19 sediment and releasing the contaminants into the water column, resulting in the transport of the sorbed  
20 contaminants over wide areas. In addition, while bioturbation by burrowing organisms may promote  
21 mixing and dilution of contaminated sediment with newly deposited cleaner sediment, for  
22 bioaccumulative contaminants it may also result in continued bioaccumulation into the food web until  
23 contaminant isolation occurs. Storm events and bioturbation may limit the long-term effectiveness of  
24 natural sedimentation, and uncertainties regarding their effects may increase the uncertainty of MNR  
25 based on sedimentation. Sediment stability is discussed in more detail in Chapter 2, section 2.5.  
26

27 There are a variety of methods to assess site-specific sedimentation rates. These methods range  
28 from direct measurement of the sedimentation rate in specially constructed traps to indirect inference of  
29 sedimentation rates using vertical concentration gradients of known contaminant releases or natural or  
30 man-made geochemical tracers that are measured in sediment core samples. Information concerning the  
31 use of radioactive tracers is included in Appendix C of this guidance.  
32

33 Methods to assess other sediment physical processes include empirical approaches such as  
34 changes in bathymetry through time, observations of bed forms and sedimentary features in core samples  
35 or sediment profile imagery, and aerial distribution of grain size (sediment trend analysis) as discussed in  
36 Chapter 2, section 2.5, Sediment Stability. Physical processes may also be assessed through modeling.  
37 Sediment transport modeling and other uses for models are discussed in Chapter 2, section 2.6, Modeling.  
38 Selection of an appropriate technical assessment approach should be based upon site-specific physical  
39 factors and the site conceptual model.  
40

### 41 **4.3.2 Biological Processes**

42

43 In biological processes such as biodegradation (or the more general term of biotransformation), a  
44 chemical change is facilitated by microorganisms living in the sediment. Biological processes depend on  
45 site-specific conditions and are highly variable. One of the important limitations to the usefulness of  
46 biodegradation as a risk-reduction mechanism is that the greater the molecular weight of the organic  
47 contaminants, the greater partitioning to sorption sites on sediment particles (Mallhot and Peters 1988)

1 and the lower the contaminant availability to microorganisms. Degradation of high molecular weight  
2 organic compounds occurs naturally, but slowly, in soil and sediment with anaerobic and aerobic  
3 microorganisms (Brown et al. 1987, Abramowicz and Olsen 1995, Bedard and May 1996, Shuttleworth  
4 and Cerniglia 1995, Cerniglia 1992, Seech et al. 1993). Degradation rates vary with depth in sediment  
5 partly due to the change from aerobic or anaerobic conditions. This changes occurs at depths of a few  
6 millimeters to a few centimeters where sediments have substantial organic content and conditions are  
7 quiescent. Longer residence times of contaminants in the sediment (aging) also usually results in  
8 increased sequestration (Luthy et al. 1997, Dec and Bollag 1997). These processes reduce the availability  
9 of the organic compounds to microorganisms and, therefore, reduce the extent and rates of biodegradation  
10 (Luthy et al. 1997, Tabak and Govind 1997).

11  
12 The class of hydrocarbons known as polynuclear aromatic hydrocarbons (PAHs) biodegrade  
13 more quickly through aerobic processes, although the degradation rates usually decrease as the number of  
14 aromatic rings increases (Shuttleworth and Cerniglia 1995, Cerniglia 1992, Seech et al. 1993). While  
15 biodegradation of PAHs may occur under anaerobic conditions, PAHs usually persist longer in anaerobic  
16 sediment compared to aerobic environments (U.S. EPA 1996e, Safe 1980).

17  
18 The composition of polychlorinated biphenyl (PCB) mixtures that occur in the environment may  
19 be quite different from those of the original mixtures released to the environment. This is a result of the  
20 combined effects of such processes as differential volatilization, solubility, sorption, anaerobic  
21 dechlorination, and metabolism, and results in changes in the composition of the PCB mixture over time  
22 and between trophic levels (NRC 2001).

23  
24 Highly chlorinated congeners of PCBs and other chlorinated contaminants may gradually  
25 dechlorinate naturally in anaerobic sediment (Brown et al. 1987, Abramowicz and Olsen 1995, Bedard  
26 and May 1996). In general, less chlorinated PCBs and other partially dechlorinated organic species tend  
27 to bioaccumulate less than the highly chlorinated congeners or species, but are more soluble and  
28 therefore more readily transported in the water column than highly chlorinated PCBs. The less  
29 chlorinated PCBs exhibit significantly less potential human carcinogenic and dioxin-like (coplanar  
30 structure) toxicity (Abramowicz and Olsen 1995, Safe 1992), but may be transformed in humans into  
31 forms with potential for other toxicity (Bolger 1993). Aerobic processes may then biodegrade the less  
32 chlorinated PCB congeners and other lightly chlorinated organics (Flanagan and May 1993, Harkness et  
33 al. 1993). The sediment concentrations of other chemicals and the total organic content tend to control  
34 these processes. However, little evidence exists that lower chlorinated congeners under the anaerobic or  
35 anoxic conditions found in most sediment are significantly transformed. Therefore, these partially  
36 dechlorinated organics tend to accumulate and persist (U.S. EPA 1996e, Harkness et al. 1993).

37  
38 Chlorinated pesticides and industrial chlorinated organics may also be transformed or partially  
39 degraded in sediment. However, the degradation products may be equally or more toxic and persistent  
40 than the original pesticide or chlorinated organic. For example, dichloro-diphenyl-trichloroethane (DDT)  
41 may be transformed under anaerobic conditions to dichloro-diphenyl-dichloroethane (DDD) and under  
42 aerobic conditions to dichloro-diphenyl-dichloroethylene [(DDE), Johnsen 1976, Rochkind and  
43 Blackburn 1986). Although all three DDT constituents may be found in sediment, DDE may be the  
44 constituent of the DDT-related compounds most widely detected (U.S. EPA 1992c, Stull et al. 1996) and  
45 most resistant to further biotransformation (NRC 1977).

1           The role of biological processes in MNR is summarized by the NRC (2001): “Of the natural-  
2 attenuation processes, biodegradation is generally considered the most desirable because it can result in  
3 elimination of risk.” While desirable, it is unclear whether biologically mediated dechlorination of  
4 pesticides and PCBs would be effective in achieving remedial objectives in a reasonable time frame and  
5 may result in the production of more toxic byproducts. Although aerobic biological degradation of PAHs  
6 has been demonstrated in the laboratory, the low levels of dissolved oxygen typically found in natural  
7 sediment limit the potential for substantial degradation of PAHs in the field.  
8

### 9   **4.3.3 Chemical Processes**

10           Many environmental variables govern the chemical state of metals in sediment, which in turn  
11 affects their mobility, toxicity, and bioavailability making natural recovery due to chemical processes  
12 difficult to predict. Much of the current understanding of the role of chemical processes in controlling  
13 risk is focused on the important geochemical changes in bioavailability of metal and organic metal  
14 compounds with changes in redox potential. Formation of relatively insoluble metal sulfides under  
15 reducing conditions often can effectively control the risk posed by metal contaminants if reducing  
16 conditions are maintained. Environmental variables include pore water pH and alkalinity, sediment grain  
17 size, oxidation-reduction (redox) conditions, and the amount of sulfides and organic carbon present in the  
18 sediments. Furthermore, many chemical processes in sedimentary environments are biologically  
19 mediated. For example, if mercury is present in sediments, formation of methylated mercury can increase  
20 bioaccumulation risk by increasing the mobility and the bioaccumulation potential of mercury under both  
21 anaerobic and aerobic conditions (Bisogni and Lawrence 1973).  
22  
23

## 24   **4.4 LINES OF EVIDENCE**

25           An evaluation of MNR as a remedy or remedy component should be thoroughly and adequately  
26 supported with site-specific characterization and analysis and verified through long-term monitoring and  
27 modeling. The SAB review (U.S. EPA 2001i) noted that: “Considerable effort will be required to  
28 establish appropriate implementation of MNA at contaminated sediment sites. Although information on  
29 contaminant attenuation in soils and ground water can serve as a basis for understanding certain process  
30 of attenuation in contaminated sediments, the dynamic and heterogeneous nature of sediment systems  
31 introduces complexities not found in soil or ground water.” Swindoll et al. (2000) include a chapter on  
32 natural remediation of sediment which presents a useful summary discussion.  
33  
34

35           An evaluation of natural processes at a site should be based on site-specific data collected during  
36 different seasons over a year’s. Time-series data from a number of years are frequently needed as well.  
37 Lines of evidence or types of site-specific information used to construct a plausible case for the use of  
38 MNR include the following:  
39

- 40           • Documentation that any ongoing sources of contamination have been controlled;
- 41           • Time series data for sediment and biota that demonstrate a clear and meaningful trend of  
42 decreasing contaminant concentration, mass, or toxicity over time;
- 43           • Core data demonstrating a predictable rate of sedimentation or contaminant  
44 transformation;
- 45           • Core data demonstrating a predictable rate of sedimentation or contaminant  
46 transformation;
- 47

- 1 • Historical information concerning frequency and intensity of natural and man-made  
2 disruptive events;
- 3
- 4 • Reliable future predictions (through modeling or other methods) of contaminant levels in  
5 surface water, sediment, and biota that demonstrate how human and ecological risks will  
6 be reduced to acceptable levels in an acceptable time frame;
- 7
- 8 • Reliable future predictions concerning natural and man-made disruptive events, their  
9 frequency, intensity, and impact on contaminant migration;
- 10
- 11 • Data from field studies (e.g., core data) that demonstrate the depth of the sediment mixing  
12 zone or zone that is currently bioavailable or likely to become so in the future;
- 13
- 14 • Data from field studies (conducted in or with actual contaminated site media and  
15 representative of site-wide exposure) that directly demonstrate the occurrence of a  
16 particular attenuating process at the site and its ability to degrade the contaminants of  
17 concern or reduce the risk of exposure to the contaminants;
- 18
- 19 • Development of conceptual and predictive models to assess current and future human and  
20 ecological exposures from the contaminated sediment; and
- 21
- 22 • Knowledge of future plans for use and development of the site and watershed.
- 23

24 The amount of physical, biological, and chemical processes information needed to adequately  
25 assess the applicability of MNR is site-specific. Integration of the data quality objective (DQO) process  
26 with the risk evaluation process should identify which natural processes are most critical to the evaluation  
27 of MNR at a site. Generally, the identification of MNR data needs and preparation of study design can be  
28 structured similarly to the DQO process (U.S. EPA 2000e) which is normally integrated within the RI/FS.  
29 The DQO process is intended to help site managers plan to collect data of the right type, quality, and  
30 quantity to support defensible site decisions. Initial key steps in the DQO processes can help assure that  
31 only data critical to the MNR decision are collected. For example these steps could include the  
32 following:

- 33
- 34 • State the problem: There is current and future exposure to humans through contaminant  
35 concentrations in fish tissue and ecological exposure to a contaminated sediment-based  
36 food web;
- 37
- 38 • Identify the decision: Natural physical, biological, and/or chemical processes are  
39 sufficient to reduce risk to acceptable levels over a defined time frame; and
- 40
- 41 • Identify inputs to the decision: What are acceptable risk levels of contaminants in the  
42 biologically active zone and the deeper zones susceptible to disruption? What is the  
43 current accretion rate? What are the biological and chemical degradation rates? How  
44 long will these processes require to reduce sediment concentrations to acceptable levels.
- 45

46 EPA guidance on the DQO process describes in detail all of the seven steps and provides  
47 examples of how each step can be applied (U.S. EPA 2000e). Data needs for evaluation of MNR can be

1 logically coordinated via the DQO process with risk assessment and other data needs of the site  
2 characterization.

#### 4 4.5 ENHANCED NATURAL RECOVERY

5  
6 In some areas, natural recovery appears to be the most appropriate cleanup method, yet, the rate  
7 of sedimentation or other natural processes is insufficient to reduce risks within an acceptable time frame.  
8 Where this is the case, project managers should consider accelerating the recovery process by the addition  
9 of a thin layer of clean sediment or by some other method of augmenting natural recovery.

10  
11 Thin-layer placement accelerates natural recovery by adding a layer of clean sediment over  
12 contaminated sediment. The acceleration can occur through several processes, including increased  
13 dilution through bioturbation of clean sediment mixed with underlying contaminants. Thin-layer  
14 placement is different than the isolation caps discussed in Chapter 5, In-situ Capping, because it does not  
15 provide long-term isolation of contaminants from benthic organisms. While thickness of an isolation cap  
16 can range up to several feet, the thickness of a thin layer placement could be as little as a few inches.  
17 The grain size and organic carbon content of the clean sediment to be used for thin layer placement  
18 should be carefully considered in consultation with aquatic biologists. The clean sediment should be  
19 selected based on its ability to provide habitat to support a healthy community of bottom-dwelling  
20 organisms and submerged aquatic vegetation.

21  
22 Clean sediment can be placed in a uniform thin layer over the contaminated area or it can be  
23 placed in berms or windrows, allowing natural sediment transport processes to distribute the clean  
24 sediment to the desired areas. Placement methods would be similar to those used for a cap. Several  
25 numerical models are available to assist in developing a strategy for thin layer placement. The Short-  
26 Term Fate (STFATE) model can be used to estimate the depositional pattern using various placement  
27 methods and source materials (USACE 1992, USACE and U.S. EPA 1998). The USACE also has a  
28 Suspended Sediment Fate model (SSFATE) that is useful. Subsequent movement of windrow-distributed  
29 material can be predicted using the Long-Term Fate (LTFATE) model (USACE 1995).

#### 31 4.6 CONCLUSIONS

32  
33 The long-term effectiveness of MNR is usually dependent on the following: 1) the likelihood for  
34 overlying clean sediment to remain in place; 2) the likelihood for transport of contaminants through  
35 overlying clean sediment to the biologically active zone or the water column; and 3) the effectiveness of  
36 source control. Assessment of all three is important at most sites. Some potential mechanisms for  
37 physical disruption of overlying sediment such as propeller wash, keel drag, navigational dredging, and  
38 pipeline construction, may be amenable to human management. Others, such as ice scour or flooding,  
39 may only be partly manageable or not manageable. This is discussed further in Chapter 2, section 2.5,  
40 Sediment Stability, of this guidance. The importance of contaminant movement through overlying  
41 sediment depends on the chemical characteristics of the contaminant, physical characteristics of the  
42 sediment, and patterns of ground water flow. This issue is also of concern for in-situ capping and is  
43 discussed further in Chapter 5, In-Situ Capping, and the USACE Technical Note, *Subaqueous Capping  
44 and Natural Recovery: Understanding the Hydrogeologic Setting at Contaminated Sediment Sites*  
45 (USACE 2002).  
46

1 MNR is likely to be effective most quickly in stable depositional environments where  
2 contaminant exposures are mostly due to recent releases from sources other than the sediment. Where  
3 external sources were controlled several decades previously and no further source control opportunities  
4 have been identified, yet site risks remain unacceptable, it may be questionable whether natural processes  
5 alone will reduce risks significantly in the future. For MNR, as for other sediment cleanup methods,  
6 effective source control is critical to reaching remedial objectives in a reasonable time frame and to  
7 preventing re-contamination.

8  
9 Similar to EPA's policy for MNA of ground water and soil, MNR for sediment should generally  
10 be used as one component of an overall site remedy and cautiously as the sole cleanup method at a  
11 contaminated sediment site. MNR should usually be used either in conjunction with source control or  
12 active sediment remediation or as a follow-up measure to an active cleanup method. For example,  
13 dredging or capping might be selected to address the most contaminated hot spot areas that pose the  
14 greatest risk, and MNR could be selected to address more widespread, low-level contamination in  
15 remaining areas that contribute to the residual site-wide risk.

16  
17 When considering the use of MNR as a follow-up measure, project managers should consider the  
18 conditions caused by the active cleanup method. As noted by the SAB (U.S. EPA 2001i): "If MNA [or,  
19 as used in this guidance, MNR] is to be considered after a remedial action (e.g., the removal of heavily  
20 contaminated portions or capping), the effects of the remedial action on the chemistry, biology, and  
21 physics of contaminated sediments should be evaluated. The effects include: 1) potential disturbances on  
22 reaction conditions and aquatic life when dredging is used, and 2) changes on reaction conditions and  
23 mass transfer in the sediment and at the sediment/water interface when capping is used."

24  
25 MNR should be considered when it would meet remedial objectives within a time frame that is  
26 reasonable compared to active cleanup methods. However, the Agency recognizes that MNR usually  
27 takes longer to reach cleanup levels in sediment than dredging or in-situ capping and, therefore, usually  
28 takes longer to reach all remedial action objectives, such as contaminant reductions in fish. Factors that  
29 the project manager should consider in determining whether the time frame for MNR is "reasonable" in  
30 comparison to the time frame needed for active cleanup methods include the following:

- 31  
32
- 33 • The extent of human exposure to contaminants during the recovery period, and if  
34 controlled by institutional controls, the effectiveness of those controls;
  - 35 • The value of ecological resources that may be impacted by contaminant releases during  
36 the recovery period;
  - 37 • The societal, cultural, and economic resources affected during the recovery period;
  - 38 • The time frame in which affected portions of the site may be needed for future uses  
39 which will be available after MNR has achieved cleanup levels; and
  - 40 • The uncertainty associated with the prediction
- 41  
42  
43  
44  
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46 Project managers should carefully consider the need for contingency measures, contingency  
47 remedies, or interim decisions where there is significant uncertainty about the effectiveness of MNR.

1 Project managers should carefully consider how risks can be controlled during the recovery period. In  
2 most cases, no institutional controls can protect ecological exposure during this period. Although  
3 institutional controls exist to protect human exposure, some, such as fish consumption advisories, cannot  
4 be expected to be fully effective and may be unenforceable. See Chapter 3, section 3.5, Institutional  
5 Controls, and Chapter 7, section 7.4, Considering Institutional Controls, for more information concerning  
6 institutional controls at sediment sites. Highlight 4-3 lists some important points to remember from this  
7 chapter.

8  
9  
10 **Highlight 4-3: Points to Remember When Considering Monitored Natural Recovery**

- 11 • Source control generally should be implemented to prevent re-contamination
- 12 • MNR frequently includes multiple physical, biological, and chemical mechanisms that act together to  
13 affect risk
- 14 • Evaluation of MNR should be based on site-specific data collected repeatedly over a number of years,  
15 including an assessment of seasonal variation
- 16 • Project managers should evaluate the long-term stability of the sediment bed and the likely ecological and  
17 human health impacts of disruption
- 18 • Multiple lines of evidence are frequently needed to evaluate MNR (e.g., time-series data, core data,  
19 modeling)
- 20 • Thin layer placement of clean sediment may accelerate natural recovery in some cases
- 21 • Institutional controls such as fish consumption advisories are of variable effectiveness and should be  
22 relied upon with caution
- 23 • Contingency measures should be included as part of an MNR remedy when there is significant  
24 uncertainty that the remedial action objectives will be achieved within the predicted time frame
- 25 • MNR should generally be used as one component of an overall site remedy, and cautiously as the sole  
26 cleanup method

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## 5.0 IN-SITU CAPPING

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## 5.0 IN-SITU CAPPING

### 5.1 INTRODUCTION

In-situ capping refers to the placement of a subaqueous covering or cap of clean material over a deposit of contaminated sediment. Caps are generally constructed of granular material, such as clean sediment, sand, or gravel. A more complex cap design can include geotextiles, liners, and multiple layers as well as additions of material to attenuate the flux of contaminants (e.g., organic carbon). Depending on the contaminants and sediment environment, a cap reduces risk through the following primary functions:

- Physical isolation of the contaminated sediment from the aquatic environment;
- Stabilization/erosion protection of contaminated sediment, preventing resuspension and transport to other sites; and
- Chemical isolation/reduction of the movement of dissolved and colloiddally transported contaminants.

Caps may be designed with different layers to serve these primary functions or in some cases a single layer may serve multiple functions.

In-situ capping has been selected as a cleanup method for contaminated sediment at about a dozen Superfund sites as of 2001. Most frequently, although not always, this method is combined with sediment removal (i.e., dredging or excavation) or monitored natural recovery of other sediment site areas. In general project managers should evaluate each of the three major cleanup methods: monitored natural recovery, in-situ capping, and removal through dredging or excavation, at every sediment site at which they may be appropriate.

Variations of in-situ capping include installation of a cap after partial removal, through dredging or excavation, of contaminated sediment, and innovative caps which incorporate treatment components. Capping is sometimes considered following partial sediment removal where capping alone is not feasible due to hydraulic or navigation restrictions on the waterway depth, or where it is desirable to leave deeper contaminated sediment in place to preserve bank or shoreline stability following hot spot removals. Backfill of clean material which is designed to mix with dredging residuals rather than act as an engineered cap to isolate buried contaminants is not considered in-situ capping in this guidance. There are a number of pilot studies underway to investigate the types of in-situ caps which incorporate various forms of treatment (see Chapter 3, section 3.1.3, In-Situ Treatment Alternatives).

Much has been written about subaqueous capping of contaminated sediment. The majority of this work has been performed by, or in cooperation with, the U.S. Army Corps of Engineers (USACE). Comprehensive technical guidance on in-situ capping as a cleanup method for contaminated sediment can be found in the U.S. Environmental Protection Agency's (EPA) *Assessment and Remediation of Contaminated Sediment (ARCS) Program Guidance for In-Situ Subaqueous Capping of Contaminated Sediments* (U.S. EPA 1998d) and the *Assessment and Remediation of Contaminated Sediments (ARCS) Program Remediation Guidance Document* (U.S. EPA 1994b). Unless an effective treatment component is incorporated into the cap, in-situ capping generally does not meet CERCLA §121(b)(1) preference for

1 treatment. When contaminants left in place are above levels that allow for unlimited use and unrestricted  
2 exposure, a five-year review generally is necessary (U.S. EPA 2001h).

3  
4 Highlight 5-1 lists some of the general site conditions which may be appropriate for consideration  
5 of in-situ capping.

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8 **Highlight 5-1: General Site Conditions Appropriate for In-Situ Capping**

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- Sources are controlled
  - Contaminants are not both, highly toxic and at high concentrations
  - Suitable types and quantities of cap materials are available
  - Hydrologic conditions (e.g., floods, ice scour) will not compromise the cap or can be accommodated in design
  - Water depth is adequate to accommodate cap with anticipated uses (e.g., navigation, flood control)
  - Ground water flow gradients are low or contaminants of low mobility
  - Weight of the cap can be supported by the underlying sediment without slope failure
  - Anticipated infrastructure needs (e.g., piers, pilings, buried cables) are compatible with cap
  - Habitat disruption is outweighed by long-term risk reduction

27  
28 **5.2 ADVANTAGES AND DISADVANTAGES**

29  
30 Two advantages of in-situ capping are that it can quickly reduce exposure to contaminants and  
31 that, unlike dredging or excavation, less infrastructure in terms of material handling, dewatering,  
32 treatment and disposal is needed. A well-designed and well-placed cap should provide a clean substrate  
33 for re-colonization by bottom-dwelling organisms. Another advantage is that the potential for  
34 contaminant resuspension and the risks associated with dispersion and volatilization of contaminated  
35 materials during construction are typically lower for in-situ capping than for dredging operations. Most  
36 capping projects use conventional and locally available materials, equipment, and expertise. For this and  
37 other reasons, in-situ capping may be implemented more quickly and may be less expensive than cleanup  
38 methods involving removal and disposal or treatment.

39  
40 In-situ capping may be less disruptive of communities than dredging or excavation. Although  
41 some local land facilities are usually needed for materials handling, no dewatering, treatment or disposal  
42 facilities need to be located and no contaminated materials are transported through communities.

43  
44 The major disadvantage of in-situ capping is that the contaminated sediment is left in place in the  
45 aquatic environment where contaminants may be exposed or dispersed if the cap is significantly  
46 disturbed. It is often necessary to rely on institutional controls, which can be limited in terms of  
47 effectiveness and reliability, to protect the cap.

1 Another potential disadvantage to in-situ capping may be that in some situations usable habitat  
2 may not be provided by the cap materials. To provide erosion protection, it may be necessary to use cap  
3 materials that are different from native bottom materials and thus can alter the biological community. For  
4 example, it may be desirable to select capping materials that discourage colonization by native deep-  
5 burrowing organisms to limit bioturbation. In either case, if the cap provides relatively poor habitat for  
6 the local biological community, potential mitigation requirements or natural resource damages may be  
7 incurred.

### 9 **5.3 COMPONENTS OF CAPPING TECHNOLOGY**

10 The general elements or components of an in-situ capping project include the following:

- 11 • Identifying candidate capping materials that are physically and chemically compatible;
- 12
- 13 • Assessing the bioturbation potential of local benthic organisms, and designing a cap  
14 component to physically isolate them from contaminated sediment;
- 15
- 16 • Evaluating the potential for erosion of the cap due to natural and anthropogenic  
17 disruptive forces and designing a cap system that will be stable or have acceptable losses  
18 under these conditions;
- 19
- 20 • Assessing the potential flux of sediment-associated contaminants, and designing a cap  
21 component to reduce the flux of dissolved and colloidal contaminants into the water  
22 column sufficiently to achieve and maintain remedial action objectives;
- 23
- 24 • Evaluating geotechnical considerations including consolidation of compressible materials  
25 and potential interactions and compatibility among cap components;
- 26
- 27 • Evaluating construction and placement methods, and identifying performance objectives  
28 and monitoring methods for cap placement and long-term assessment;
- 29
- 30 • Assessing the operational considerations and determining restrictions or additional  
31 protective measures (e.g., institutional controls) needed to ensure cap integrity; and
- 32
- 33 • Assessing the potential for long-term habitat alteration (e.g., changes in depth, substrate,  
34 and hydrodynamic regime) and mitigating, if appropriate.
- 35
- 36
- 37

38 The first six of these topics are discussed briefly below. General monitoring considerations for in-situ  
39 capping are discussed in Chapter 8, section 8.4.2. Institutional controls are discussed in Chapter 3,  
40 section 3.5, and habitat considerations are addressed in section 5.4.4 of this chapter.

#### 41 **5.3.1 Identification of Capping Materials**

42 Caps are generally composed of clean granular materials, such as sediment, sand, or soil;  
43 however, more complex cap designs could be required to meet site-specific remedial action objectives.  
44 As discussed below, the project manager should take into consideration the need for effective short- and  
45 long-term chemical isolation of contaminants, bioturbation, consolidation, erosion, and other related  
46  
47

1 processes. For example, if the potential for erosion of the cap is significant, the cap thickness could be  
2 increased using a material with larger grain size, or an armor layer could be incorporated into the design.  
3 Porous geotextiles do not contribute to contaminant isolation, but serve to reduce the potential for mixing  
4 and displacement of the underlying sediment with the cap material. A cap composed of naturally  
5 occurring sand is generally preferred over processed sand because the associated fine fraction and organic  
6 carbon content found in natural sands are more effective in providing chemical isolation by sequestering  
7 contaminants migrating through the cap. In some conditions it may be feasible to consider use of lightly  
8 contaminated dredged material as sub-grade cap material, if the site remedy includes both dredging and  
9 capping. Also, specialized materials may be considered for caps to enhance the chemical isolation  
10 capacity. Examples include engineered clay aggregate materials (e.g., AquaBlok™). Highlight 5-2  
11 illustrates some examples of cap designs.

### 12 **5.3.2 Physical Isolation Component**

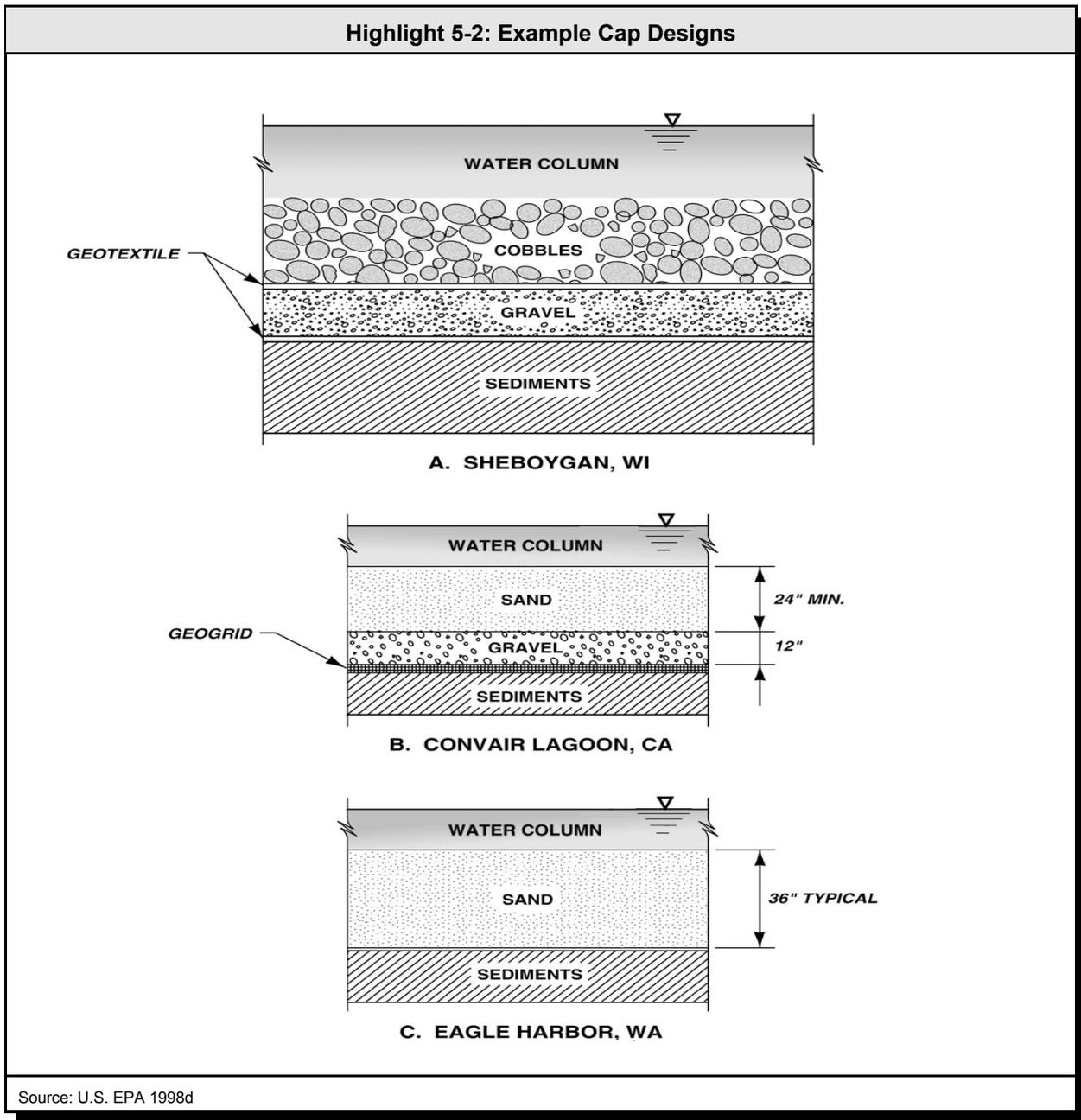
13  
14  
15 The cap should be designed to isolate sediment contaminants from the aquatic environment. The  
16 physical isolation component of the cap includes separate subcomponents for isolation and consolidation.  
17

18 To provide long-term protection, a cap should be sufficiently thick to effectively separate  
19 contaminated sediment from aquatic organisms which dwell or feed on, above, or within the cap. To  
20 design a cap component for this function, the bioturbation potential of local bottom-dwelling organisms  
21 should be evaluated. In marine environments, the potential for colonization by deep burrowing organisms  
22 (e.g., certain species of mud shrimp) could require increases in cap design thickness. Measures to prevent  
23 colonization or disturbance of the cap by deep burrowing bottom-dwelling organisms can be considered  
24 in cap design, and in developing biological monitoring requirements for the project. Project managers  
25 should refer to Chapter 2, section 2.5.3 and consult with aquatic biologists with knowledge of local  
26 conditions for evaluation of the bioturbation potential. In some cases, a site-specific biological survey of  
27 bioturbators would be appropriate. In addition, the USACE Technical Note, *Subaqueous Cap Design:  
28 Selection of Bioturbation Profiles, Depths and Process Rates* (USACE 2001), provides information on  
29 designing in situ caps and also provides many useful references on bioturbation. This document is  
30 available at <http://www.wes.army.mil/el/dots/doer/technote.html>.

31  
32 The project manager should consider consolidation when designing the cap. Fine-grained  
33 granular capping materials can undergo consolidation due to self-weight. Even if the cap material is not  
34 compressible, most contaminated sediment is highly compressible. Underlying contaminated sediment  
35 will almost always undergo consolidation due to the added weight (i.e., overburden) of the capping  
36 material or armor stone. The thickness of granular cap material should have an allowance for  
37 consolidation so that the minimum required cap thickness is maintained following consolidation.  
38

39 An evaluation of consolidation is important in interpreting monitoring data to differentiate  
40 between changes in cap surface elevation or cap thickness due to consolidation, as opposed to erosion.  
41 Also, the degree of consolidation will provide an indication of the volume of pore water that will be  
42 expelled through the contaminated layer and capping layer due to consolidation. The consolidation-  
43 driven advection of pore water should be considered in the evaluation of contaminant flux. Also,  
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consolidation may decrease the vertical permeability of the capped sediment. Methods used to define and quantify consolidation characteristics of sediment and capping materials, such as standard laboratory tests and computerized models, are available (U.S. EPA 1998d and Palermo et al. 1998a).

1 **5.3.3 Stabilization/Erosion Protection Component**  
2

3 The cap component for stabilization/erosion protection has a dual function. This component of  
4 the cap is intended to stabilize the contaminated sediment being capped, and prevent the sediment from  
5 being resuspended and transported off site. The other function of this component is to make the cap itself  
6 resistant to erosion as represented in the Sheboygan example in Highlight 5-2.  
7

8 The potential for erosion generally depends on the magnitude of the applied bed shear stresses  
9 due to river, tidal and wave-induced currents, and turbulence generated by ships/vessels (due to propeller  
10 action and vessel draft), and sediment properties such as particle size, mineralogy and bed bulk density.  
11 These and other aspects of investigating sediment stability are discussed in Chapter 2, section 2.5,  
12 Sediment Stability. Conventional methods for analysis of sediment transport are available to evaluate  
13 erosion potential of caps, ranging from simple analytical methods to complex numerical models (U.S.  
14 EPA 1998d and Palermo et al. 1998a). Uncertainty in the estimate of erosion potential should be  
15 evaluated as well.  
16

17 The design of erosion protection features of an in-situ cap (i.e., armor layers) should be based on  
18 the magnitude and probability of occurrence of extreme erosive forces estimated at the capping site. At a  
19 minimum, in-situ caps should be designed to withstand forces with a probability of 0.01 per year, for  
20 example, the 100-year storm. As is discussed further in Chapter 2, section 2.5, Sediment Stability, in  
21 some circumstances, lower probability events should also be considered.  
22

23 For sediment with high organic content (e.g., wood processing sites) significant gas generation  
24 may occur due to anaerobic degradation. Gas generation in sediment beneath the cap could add  
25 significant uplift forces on impermeable membrane and geotextiles and threaten the physical stability of  
26 the overlying capping materials. The possible influence of this process on cap effectiveness presents an  
27 uncertainty that the project manager should consider in the analysis of remedial alternatives.  
28

29 **5.3.4 Chemical Isolation Component**  
30

31 If a cap has a properly designed physical isolation component, contaminant migration associated  
32 with the movement of sediment particles should be controlled. However, the vertical movement of  
33 contaminants by advection (flow of pore water) through the cap is possible, while movement by  
34 molecular diffusion (across a concentration gradient) over long periods is inevitable. If reduction of  
35 contaminant flux is necessary to meet remedial action objectives, a more involved analysis to include  
36 capping effectiveness testing and modeling should be conducted as a part of cap design.  
37

38 Both advective and diffusive processes should be considered in cap design. If a ground  
39 water/surface water interaction study indicates that advection is not significant at a given location (e.g.,  
40 migration of ground water upward through the cap would not prevent attaining the remedial action  
41 objectives), the cap design may only need to address diffusion and the physical isolation and stabilization  
42 of the contaminated sediment. In this case, it may not be necessary to design for dissolved and/or  
43 colloiddally facilitated transport due to advection (Ryan et al. 1995). In contrast, where ground water flow  
44 upward through the cap is expected to be significant, the hydraulic properties of the cap should also be  
45 determined and factored into the cap design. These properties should include the hydraulic conductivities  
46 of the cap materials, the contaminated sediment, and underlying clean sediment or bedrock. According to  
47 a USACE laboratory study, ground water flow velocities exceeding  $10^{-5}$  cm/sec potentially result in

1 conditions in which equilibrium partitioning processes essential to cap effectiveness could not be  
2 maintained (Myers et al. 1991). Such conditions should be carefully considered in the cap design. In  
3 areas with high rates of ground water flow discharges through contaminated sediment, in-situ capping  
4 may not be an effective remedial approach without additional protective measures. More information on  
5 the interactions of ground water can be found in the USACE Technical Note, *Subaqueous Capping and*  
6 *Natural Recovery: Understanding the Hydrogeologic Setting at Contaminated Sediment Sites* (USACE  
7 2002).

8  
9 Laboratory tests can be used to define sediment- and capping material-specific diffusion and  
10 chemical partitioning coefficients. Several numerical models are available to predict long-term  
11 movement of contaminants due to advection and diffusion processes into or through caps, including caps  
12 with engineered components. The results generated by such models include flux rates and sediment  
13 contaminant pore water concentrations as a function of time. These results can be compared to applicable  
14 water quality criteria, or interpreted in terms of a mass loss of contaminants as a function of time. Results  
15 could be compared to similar calculations for other remediation technologies. The models can evaluate  
16 the effectiveness of varying thicknesses of granular cap materials with differing properties [grain size and  
17 total organic carbon (TOC)].

### 18 19 **5.3.5 Geotechnical Considerations**

20  
21 Usually, contaminated sediment is predominately fine-grained, and often has high water content  
22 and low shear strength. These materials are generally compressible. Unless appropriate controls are  
23 implemented, contaminated sediment can be easily displaced or resuspended during cap placement.  
24 Following placement, cap stability and settlement due to consolidation are two additional geotechnical  
25 issues.

26  
27 As with any geotechnical problem of this nature, the shear strength of the underlying sediment  
28 will influence its resistance to localized bearing capacity or sliding failures, which could cause localized  
29 mixing of capping and contaminated materials. Cap stability immediately after placement is critical,  
30 before any excess pore water pressure due to the weight of the cap has dissipated. Usually, gradual  
31 placement of capping materials over a large area will reduce the potential for localized failures.  
32 Information on the behavior of soft deposits during and after placement of capping materials is limited,  
33 although some field monitoring data have shown successful sand capping of contaminated sediment with  
34 low strength. Conventional geotechnical design approaches should, therefore, be applied with caution.  
35 These design approaches should be conservative for conditions normally encountered in cap design. For  
36 example, a cap should be built up gradually over the entire area to be capped. This will reduce the  
37 potential for mixing and overturning of the contaminated sediment. Similarly, caps with flat transition  
38 slopes at the edges are not generally subject to a sliding failure normally evaluated by conventional slope  
39 stability analysis.

### 40 41 **5.3.6 Placement Methods and Performance Monitoring**

42  
43 A variety of equipment types and placement methods have been used for capping projects. The  
44 use of granular capping materials (sediment and soil), geosynthetic fabrics, and armored materials are all  
45 in-situ cap considerations discussed in this section. Important considerations in selection of placement  
46 methods include the need for controlled, accurate placement of capping materials. Slow, uniform  
47 application that allows the capping material to accumulate in layers is often necessary to avoid

1 displacement of or mixing with the underlying contaminated sediment. Uncontrolled placement of the  
2 capping material can also result in the resuspension of contaminated material into the water column.  
3

4 Granular cap material can be handled and placed in a number of ways. Mechanically excavated  
5 materials and soils from an upland site or quarry have relatively little free water. These materials can be  
6 handled mechanically in a dry state until released into the water over the contaminated site. Mechanical  
7 methods (such as clamshells or release from a barge) rely on gravitational settling of cap materials in the  
8 water column, and could be limited by depth in their application. Granular cap materials can also be  
9 entrained in a water slurry and carried to the contaminated site wet, where they can be discharged by pipe  
10 into the water column at the water surface or at depth. These hydraulic methods offer the potential for a  
11 more precise placement, although the energy required for slurry transport could require dissipation to  
12 prevent resuspension of contaminated sediment. Armor layer materials can be placed from barges or  
13 from the shoreline using conventional equipment, such as clamshells. Placement of cap components, such  
14 as geotextiles, could require special equipment. Examples of equipment types used for cap placement are  
15 shown in Highlight 5-3. The *Guidance for In-Situ Subaqueous Capping of Contaminated Sediments*  
16 (U.S. EPA 1998d) contains more detailed information about cap placement techniques.  
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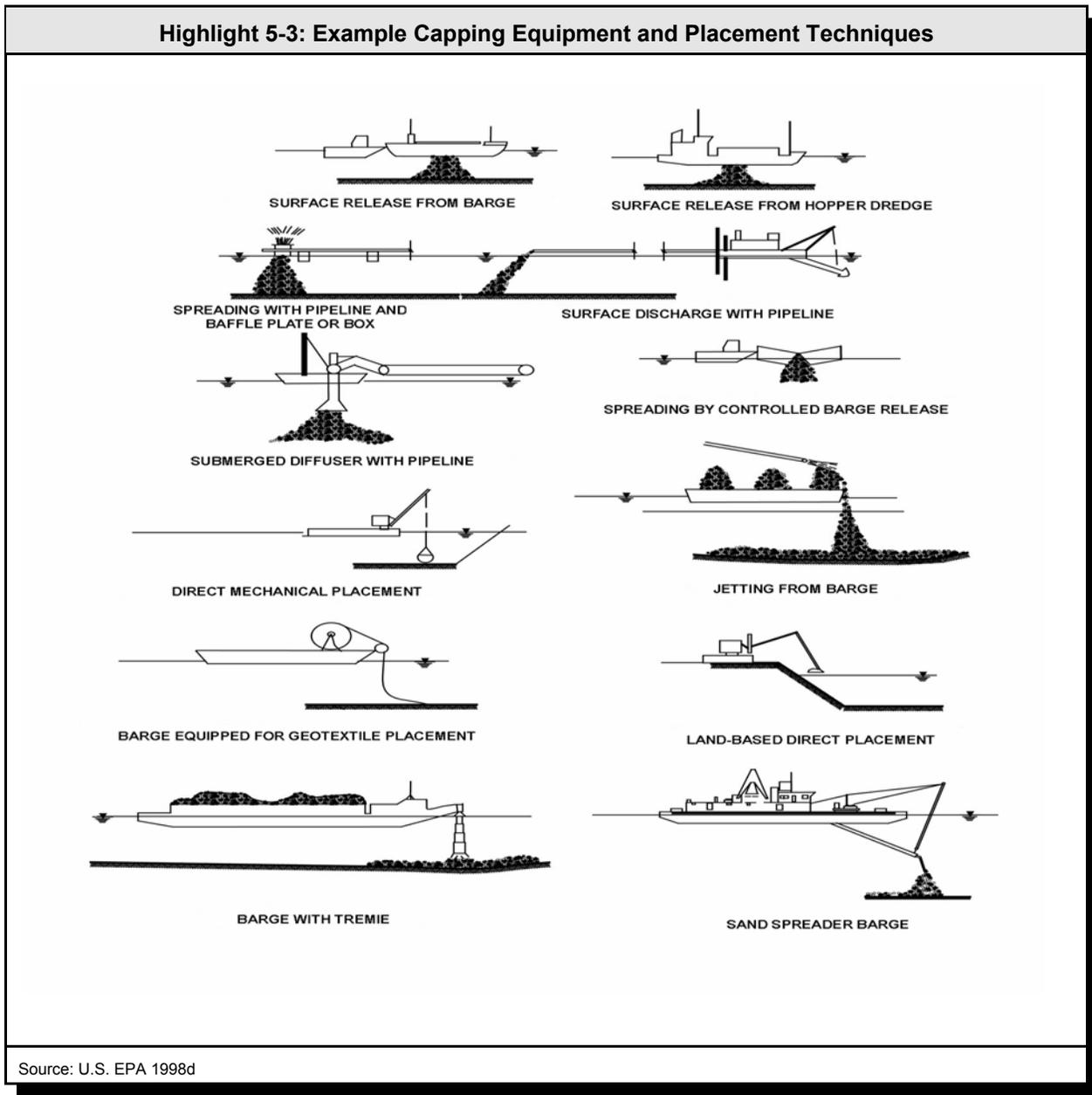
18 Monitoring of both placement and performance of in-situ caps are important. Cap placement can  
19 resuspend some contaminated sediment, leading to contaminant release and increased risks during and  
20 following cap placement. Therefore, designs should include plans to mitigate and monitor impacts during  
21 and after construction.  
22

23 Performance objectives for an in-situ cap relate to its ability to provide sufficient physical  
24 isolation and stabilization of contaminated sediment. Broader remedial action objectives such as  
25 decreases in contaminant concentrations in biota also should be evaluated when applicable, and are discussed  
26 in Chapter 8, Remedial Action and Long-Term Monitoring. The following processes should be  
27 considered when evaluating the performance of a cap, and in developing a cap design and monitoring  
28 program:  
29

- 30 • Upward contaminant flux rates (mass of contaminant/unit area/unit time);
- 31
- 32 • Movement of contaminants into pore water of cap;
- 33
- 34 • Changes in chemical environment (e.g., redox potential) that may change mobility of  
35 contaminants;
- 36
- 37 • Long-term accumulation of contaminants in cap material;
- 38
- 39 • Contaminant breakthrough as a function of time; and
- 40
- 41 • Ability of the cap to withstand bioturbation and erosive forces.  
42

43 For example, contaminant flux and the resulting contaminant concentration in surface sediment,  
44 cap pore water, overlying surface water quality or underlying ground water quality can be compared to  
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Source: U.S. EPA 1998d

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site-specific sediment cleanup levels or water quality standards (e.g., federal water quality criteria or state promulgated standards). In addition, the concentration of contaminants accumulating in the cap material as a function of time can be compared to site-specific target cleanup levels during long-term cap performance monitoring. Both analytical and numerical models exist to predict cap performance and have been compared and validated with laboratory tests and field results (e.g., Ruiz et al. 1999).

1 **5.4 EVALUATING SITE CONDITIONS**  
2

3 A good assessment of site-specific conditions is critical to the feasibility and effectiveness of in-  
4 situ capping. Site conditions affect all aspects of a capping project, including design, equipment and cap  
5 material selection, and monitoring and management programs. Some limitations in site conditions can be  
6 accommodated in the cap design. A thorough examination of site conditions should determine if further  
7 consideration of capping is appropriate. General aspects of site characterization are discussed in Chapter  
8 2, Remedial Investigation Considerations. Some specific aspects of site characterization important for in-  
9 situ capping are introduced briefly below.

10 **5.4.1 Physical Environment**  
11

12  
13 Some considerations concerning the physical environment of an in-situ cap include the waterbody  
14 dimensions, depth and slope (bathymetry) of sediment bed, and flow patterns, including tides, currents,  
15 and other potential disturbances. Existing infrastructure such as bridges, utility crossings, and other  
16 marine structures are discussed in section 5.4.3.

17  
18 The bathymetry of the site influences the amount of spread during placement and the stability of  
19 capping material. Flatter bottom slopes allow material to be placed more accurately, especially if capping  
20 material is to be placed hydraulically. Water depth also influences the amount of spread during cap  
21 placement. Generally, the longer the descent of the cap material through the water column, the more  
22 water is entrained in the plume, resulting in a thinner layer of cap material over a larger area than  
23 intended.

24  
25 The energy of water flows is also an important consideration. Capping projects are easier to  
26 design in low energy environments (e.g., protected harbors, slow-flowing rivers, or micro-tidal estuarine  
27 systems). In open water, deeper sites are generally less influenced by wind or wave generated currents  
28 and less prone to erosion than shallow, near-shore environments. However, armoring techniques or  
29 selection of erosion-resistant capping materials can make capping technically feasible in some high  
30 energy environments. Currents within the water column affect dispersion during cap placement and can  
31 influence the selection of the equipment to be used for cap placement. Bottom currents generate shear  
32 stresses which act on the cap surface and may potentially erode the cap. In addition to ambient currents  
33 due to normal riverine or tidal flows, the project manager should consider the effects of storm-induced  
34 waves and other episodic events (e.g., floods, ice scour).

35  
36 The presence of an in-situ cap can alter existing hydrodynamic conditions. In harbor areas or  
37 estuaries, the decrease in depth or change in bottom geometry can affect the near-bed current patterns, and  
38 thus the flow-induced bed shear stresses. In a riverine environment, the placement of a cap generally  
39 reduces depth and restricts flow and may alter the sediment carrying capacity of the channel. Modeling  
40 studies may be useful to assess these changes in site conditions. In depositional areas, the effect of new  
41 sediment deposited on the cap should be considered. Clean sediment accumulating on the cap can  
42 increase the isolation effectiveness of the cap over the long term and may also increase consolidation of  
43 underlying sediment bed.  
44

1 **5.4.2 Sediment Characteristics**  
2

3 The project manager should determine the physical, chemical, and biological characteristics of  
4 the contaminated sediment pursuant to the data quality objective (DQO) process during the remedial  
5 investigation. The results of the characterization, in agreement with the remediation goals and objectives,  
6 determines the areal extent or boundaries of the area to be capped.  
7

8 Shear strength of contaminated sediment deposits is of particular importance in determining the  
9 feasibility of in-situ capping. Most contaminated sediment is fine-grained, and is usually high in water  
10 content and relatively low in shear strength. Although a cap can be constructed on sediment with low  
11 shear strengths, the ability of the sediment to support a cap and the need to construct the cap using  
12 appropriate methods to avoid displacement of the contaminated sediment should be carefully considered.  
13 The presence of other materials within the sediment bed, such as debris, wood chips or high sludge  
14 fractions, or other non-mineral-based sediment fractions, can also present special problems when  
15 interpreting grain size and other geotechnical properties of the sediment. It could be necessary to remove  
16 large debris prior to placing a cap.  
17

18 The chemical characteristics of the contaminated sediment is an important factor that may affect  
19 design or selection of a cap, especially if capping highly mobile or highly toxic sediment. In addition,  
20 capping may change the contaminated sediment from an oxidizing to an anoxic condition, which may  
21 change the solubility of metal contaminants and the susceptibility of organic contaminants to microbial  
22 decomposition. For example, many of the divalent metal cations (e.g., lead, nickel, zinc) become less  
23 soluble in anaerobic conditions, while other metal ions (e.g., arsenic, copper) become more soluble.  
24 Metals solubility is also greatly affected by pH, and so varies between fresh-water and marine systems.  
25 Mercury becomes methylated through the action of anaerobic bacteria and highly chlorinated PCBs may  
26 degrade to less chlorinated forms in an anaerobic environment. These issues are also discussed in  
27 Chapter 4, sections 4.3.2 and 4.3.3.  
28

29 When contaminated materials or sludge containing concentrated organic matter are capped, the  
30 organic matter may be decomposed by anaerobic microorganisms. The products of this decomposition  
31 process may include methane and hydrogen sulfide gases. As these dissolved gases accumulate and  
32 transfer into a gaseous phase they could begin to percolate through the capped matrix by convective or  
33 diffusive transport. This transport of gases percolating through the cap can facilitate a more rapid  
34 contaminant migration (than that due to diffusion) by providing avenues for contaminant release or  
35 solubilizing the contaminants of concern and carrying them through the saturated porous media dissolved  
36 in the gaseous molecules. These factors should be considered during cap design.  
37

38 **5.4.3 Waterway Uses and Infrastructure**  
39

40 If the site under consideration is adjacent to or within a navigation, recreation or flood control  
41 channel, the effect of cap placement on those functions of the channel should be evaluated. Effects of cap  
42 placement on habitat function of a waterbody are discussed in the following section. Cap placement may  
43 have the beneficial effect of restoring habitat, but it also decreases the water depth and cross-sectional  
44 area of the channel. The flow carrying capacity of a channel and the navigable depth could also be  
45 reduced. If water depths are reduced in a harbor or river channel, some commercial and recreational  
46 vessels may have to be restricted or banned. The acceptable draft of vessels allowed to navigate over a  
47 capped area depends on water level fluctuations (seasonal, tidal, and wave) and the potential effects of

1 vessel groundings on the cap. Due to potential cap erosion caused by propeller wash, engine size  
2 restrictions could also be needed. Generally, anchoring should not be allowed at locations on or near a  
3 cap. It may be necessary to restrict fishing and swimming to prevent recreational boaters from dragging  
4 anchors across a cap. In some situations, dredging prior to cap placement may minimize these adverse  
5 impacts of cap construction.

6  
7 Other activities in and around the waterbody may also impact cap integrity and maintenance  
8 needs, and should be evaluated. These include the following:

- 9
- 10 • Water supply intakes;
  - 11
  - 12 • Storm water or effluent discharge outfalls;
  - 13
  - 14 • Utilities and utility crossings;
  - 15
  - 16 • Construction of bulkheads, piers, docks, and other waterfront structures;
  - 17
  - 18 • Navigational dredging adjacent to the cap area; and
  - 19
  - 20 • Future development of commercial navigation channels in the vicinity of the cap.
  - 21

22 Utilities (e.g., storm drains) and utility crossings (e.g., water, sewer, gas, oil, telephone, cable, and  
23 electric) are commonly located in urban waterways. It may be necessary to relocate existing utility  
24 crossings under portions of waterbodies if their deterioration or failure might impact cap integrity or if  
25 they could not be repaired without disturbing the cap. Future construction or maintenance of utility  
26 crossings would have to consider the cap, and it may be necessary to consider limiting those activities  
27 through institutional controls if cap repair cannot be assured. The presence of the cap can also place  
28 constraints on future waterfront development that could require dredging in the area.

29  
30 To date, environmental agencies have little experience with the ability to enforce use restrictions  
31 necessary to protect the integrity of an in-situ cap (e.g., vessel size limits, bans on anchoring, etc.),  
32 although experience is growing. Generally, a local enforcement mechanism is necessary to control  
33 specific use restrictions. Project managers should consider mechanisms for compliance assurance,  
34 enforcement, and the consequences of non-compliance, on use restrictions when evaluating in-situ  
35 capping.

#### 36 37 **5.4.4 Habitat Alterations**

38  
39 In-situ capping alters the aquatic environment and, therefore, affects aquatic organisms. Habitat  
40 considerations are especially important when evaluating materials for the uppermost layers of a cap.  
41 Sandy sediment and stone armor layers are often used to cap areas with existing fine-grained sediment.  
42 Although the capped areas are already affected by contamination, the new substrate can change the  
43 habitat for benthic organisms. At least initially, changes in organic carbon content of the capping  
44 material may change the feeding behavior of benthic organisms in the capped area. Generally, the  
45 uppermost cap layers become a substrate for re-colonization. Where possible, caps should be designed to  
46 provide habitat for these organisms. In some cases it is possible to provide a habitat layer over an erosion  
47 protection layer by filling the interstices of armor stones with materials such as crushed gravel. In some

1 cases, natural sedimentation processes after cap placement can create desirable habitat characteristics.  
2 For example, placement of a rock cap in some riverine systems can result in a final cap surface that is  
3 similar to that which previously existed because the rock may become embedded with sands/silts through  
4 natural sedimentation.

5  
6 Desirable habitat characteristics for cap surfaces vary by location. Providing a layer of  
7 appropriately sized rubble that can serve as hard substrate for attached molluscs (e.g., oysters or mussels)  
8 can greatly enhance the ecological value. Material suitable for colonization by foraging organisms, such  
9 as bottom-dwelling fish, can also be appropriate. A mix of cobbles and boulders may be desirable for  
10 aquatic environments in areas with substantial flow. In addition, the potential for attracting burrowing  
11 organisms incompatible with the cap design or ability to withstand additional physical disturbances  
12 should be considered. Habitat enhancements should not impair the function of the cap or its ability to  
13 survive the shear stresses of storms, floods, propeller wash, or other disturbances. Project managers  
14 should consult with local resource managers and natural resource trustee agencies to determine what types  
15 of modifications to the cap surface would provide suitable substrate for local organisms.

16  
17 Project managers should also consult EPA staff working with the Clean Water Act, as well as  
18 natural resource trustees, where Section 404 of the Clean Water Act is applicable or relevant and  
19 appropriate (see Chapter 3, section 3.3, Applicable or Relevant and Appropriate Requirements for  
20 Sediment Alternatives). Where cleanup methods are being considered which damage aquatic habitat,  
21 substantive requirements may include minimizing the permanent loss of habitat and mitigating it by  
22 creation or restoration of a similar habitat elsewhere. Habitat considerations are also important when  
23 evaluating post-capping bottom elevations. Capping often increases bottom elevations, which in itself  
24 can alter the pre-existing habitat. For example, a remediated subtidal habitat can become intertidal, or  
25 lake habitat can become a wetland (Cowardin et al. 1979).

26  
27 Highlight 5-4 presents some general points to remember from this chapter.

**Highlight 5-4: Points to Remember When Considering In-Situ Capping**

- Source control generally should be implemented to prevent re-contamination
- In-situ caps generally reduce risk through three primary functions: physical isolation, stabilization, and reduction of contaminant transport
- Caps may be most suitable where water depth is adequate, slopes are moderate, ground water flow gradients are low or contaminants not mobile, substrates are capable of supporting a cap, and an adequate source of cap material is available
- Evaluation of capping alternatives and design of caps should consider buried infrastructure, such as water and sewer lines, fuel pipelines, and telephone and cable systems
- Substrate and depth alteration from capping should be evaluated for impacts to aquatic biota
- In evaluating a capping project in natural riverine environments, the project manager should consider a fluvial system's inherent dynamics, especially the effects of channel migration and flow variability including extreme events
- Evaluation of capping alternatives should include consideration of cap disruption from human and natural sources, including at a minimum, the 100-year flood and other events with a similar probability of occurrence
- Cap placement methods should be selected to minimize the resuspension of contaminated sediment
- The use of experienced contractors skilled in marine construction techniques is very important to placement of an effective cap
- In-situ caps should be monitored during and after placement to ensure long-term integrity of the cap, recovery of biota, and test for re-contamination